



A numerical study of rotational and transverse galloping rectangular bodies

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Abstract

In this paper we investigate rotational and translational galloping instabilities due to fluid/structure interaction using a previously developed algorithm. This numerical technique utilizes a two-dimensional spectral/*hp* element method and a frame of reference transformation to ensure efficient computations. Both transverse and rotational motion of rectangular sections of varying aspect ratio are simulated for a Reynolds number of 250 and at reduced velocities which promote a galloping response. Qualitative comparisons with quasi-steady theory and experimental data are found to be favourable.

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1. Introduction

The study of flow-induced vibration of a structure is important in many fields including aeronautical, offshore, civil and mechanical engineering. The occurrence of flow-induced vibration is due to many different mechanisms, which are classified in terms of their predominant effect (Naudascher and Rochwell, 1994; Deniz, 1997). Two well-known phenomena in the problems of fluid/structure interaction are vortex-induced vibration (VIV) and galloping. VIV is associated with synchronization, or lock-in of the structural oscillation frequency with the vortex-shedding frequency, while galloping is driven by a time-averaged fluid force which develops in phase with the structural velocity and has a frequency many times lower than that of vortex shedding. Lock-in occurs at reduced velocities where the vortex-shedding frequency is comparable to the natural frequency of the structure, whereas galloping is prevalent at higher reduced velocities where the frequency of oscillation is lower than the vortex-shedding frequency.

Both mechanisms can lead to significant oscillation amplitudes, which can potentially result in catastrophic failure of a structure. A classic example of rotational galloping is the notorious collapse of the Tacoma Narrows Suspension Bridge, which was essentially due to a torsional instability (Steinman and Watson, 1957; Scanlan, 1979; Scanlan and Tomko, 1971). Transverse galloping oscillations can also be witnessed in winter when the build up of ice on long powerlines leads to the cables undergoing large amplitude, vertical oscillations.

Extensive work has been undertaken towards the investigation of the lock-in phenomena, which has been predicted by numerous numerical codes (Nomura, 1993; Wei et al., 1995; Anagnostopoulos, 1994; Schulz and Kallinderis, 1998; Piperno, 1998; Li et al., 2002). Conversely, galloping instability has been the subject of very little reported research using numerical simulation.

The use of quasi-steady theory to predict whether a structure has the potential to gallop has been widely used for many years. It was developed by den Hartog (1956) and has generally been implemented to predict the critical reduced

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velocity for the onset of translational galloping instabilities. This has been verified experimentally by a number of investigators (Parkinson, 1971; Blevins, 1990; Nakamura and Tomonari, 1977; Nakamura and Nakashima, 1986; Nakamura and Mizota, 1975). In this paper quasi-steady theory is used to determine the conditions under which rectangular structures of varying cross sectional area will undergo translational and rotational galloping and these predictions are compared with computationally derived results.

In Section 2 of the paper the solution method is outlined. Section 3 summarizes the aerodynamic and structural equations and governing parameters. Section 4 contains an outline of the quasi-steady formulation of the galloping instability and comparison with numerical results. The conclusions constitute the final section.

2. Solution methods

For the problem of an elastically mounted rigid body in a low speed flow, the governing equations are the incompressible Navier–Stokes equations with moving boundary conditions,

$$\nabla \cdot \mathbf{v} = 0, \quad (1)$$

$$\frac{\partial \mathbf{v}}{\partial t} + (\mathbf{v} \cdot \nabla) \mathbf{v} = -\frac{1}{\rho} \nabla p + \nu \nabla^2 \mathbf{v} \quad \text{in } D(t), \quad (2)$$

where $\mathbf{v} = \mathbf{v}_b(t)$ on $\Gamma(t)$. Here \mathbf{v} is the fluid velocity, ρ is the density of the fluid, p is pressure, $D(t)$ represents the moving fluid computational domain and $\Gamma(t)$ denotes the interface between the flow and the body and moves with an unknown velocity \mathbf{v}_b .

For a single rigid body, its planar motion in two dimensions can be described in terms of three displacement components defined at the centre of gravity (Nomura and Hughes, 1992), $\mathbf{X} = (\zeta, \eta, \theta)^T$, where ζ and η are the translational displacement components in the x and y direction, respectively, and θ denotes the rotational displacement component. We may write the governing equation for this linear, planar motion as

$$\mathbf{M}\ddot{\mathbf{X}} + \mathbf{D}\dot{\mathbf{X}} + \mathbf{K}\mathbf{X} = \bar{\mathbf{F}}, \quad (3)$$

where \mathbf{M} , \mathbf{D} and \mathbf{K} are the mass, damping and stiffness matrices, respectively, $\bar{\mathbf{F}}$ is a vector whose components are external forces and moment and a dot above a variable denotes differentiation with respect to time. The mounting system for the body is assumed to have both stiffness and damping.

Instead of directly solving the coupled system of equations (2) and (3), an efficient strategy is to solve the Navier–Stokes equations and structural equation explicitly and thereby decouple their solutions at each time step. This solution technique is commonly known as a loosely coupled algorithm and has been observed to be stable for cases where the structural density is much larger than the fluid density. Therefore, at each time level, we initially solve the Navier–Stokes equations to obtain the aerodynamic force acting on the body, then use the forces as an input in the structural solver to predict the displacement of the body at the next time level. This displacement is utilized as the boundary condition on the structure in the Navier–Stokes solver.

The most general and widely used method to simulate moving boundary problems is the arbitrary Lagrangian–Eulerian formulation (ALE) where the computational mesh at the far-field boundaries is stationary and the nodes on the moving boundary take the same velocity as the structure (Nomura and Hughes, 1992). The configuration of the computational mesh therefore changes at each time-step resulting in the Navier–Stokes solver constantly being regenerated, leading to large distortions of the numerical grid and preventing the use of efficient direct solvers for static meshes.

In Li et al. (2002), a numerical method is formulated which enables efficient computations by evaluating the governing two-dimensional aerodynamic equations (1) and (2) on a fixed mesh. These equations are calculated in a moving frame of reference which is fixed to the structure and moves rigidly with it. The relationship between this moving frame of reference and a stationary fixed frame of reference is established, which leads to additional forcing terms in the Navier–Stokes equation (2). However, the extra computational cost in evaluating these additional forcing terms is small compared to that saved by evaluating the equations on a static mesh.

3. Model equations

We will consider the non dimensional form of the Navier–Stokes equations:

$$\nabla^* \cdot \mathbf{v}^* = 0, \quad (4)$$

$$\frac{\partial \mathbf{v}^*}{\partial t^*} + (\mathbf{v}^* \cdot \nabla^*) \mathbf{v}^* = -\nabla^* p^* + \text{Re}^{-1} (\nabla^*)^2 \mathbf{v}^*. \tag{5}$$

The variables in the equations above have been nondimensionalized as follows:

$$\begin{aligned} t^* &= \frac{tU}{D}, & x^* &= \frac{x}{D}, & y^* &= \frac{y}{D}, \\ u^* &= \frac{u}{U}, & v^* &= \frac{v}{U}, & p^* &= \frac{p}{\rho U^2}, \\ \text{Re} &= \frac{\rho UD}{\mu} = \frac{UD}{\nu}, \end{aligned} \tag{6}$$

where D is a characteristic length such as the section depth, and U is the freestream velocity, respectively. Correspondingly, the forces and moment are nondimensionalized by

$$F_x^* = \frac{F_x}{\rho D U^2}, \quad F_y^* = \frac{F_y}{\rho D U^2}, \quad M_\theta^* = \frac{M_\theta}{\rho D^2 U^2}. \tag{7}$$

We note that the usual lift, drag and pitching moment coefficients are given by

$$C_x = 2F_x^*, \quad C_y = 2F_y^*, \quad C_M = 2M_\theta^*. \tag{8}$$

The governing structural equation for the one-degree-of-freedom transverse or heaving mode is

$$m\ddot{y} + c_y\dot{y} + k_y y = F_y, \tag{9}$$

where m is the mass per unit length of the body, c_y is the damping constant, k_y is the stiffness constant, and y denotes the transverse location of the centre of the body. If we use the same nondimensional scales as applied to the Navier–Stokes equations, then we obtain

$$\ddot{y}^* + 2\xi_y \left(\frac{2\pi}{U_y}\right) \dot{y}^* + \left(\frac{2\pi}{U_y}\right)^2 y^* = \frac{F_y^*}{n_y} = \frac{C_y}{2n_y}, \tag{10}$$

where

$$\begin{aligned} U_y &= \frac{U}{f_y D} \quad (\text{reduced velocity}), \\ n_y &= \frac{m}{\rho D^2} \quad (\text{mass ratio}) \end{aligned} \tag{11}$$

and

$$\begin{aligned} f_y &= \frac{1}{2\pi} \sqrt{\frac{k_y}{m}} \quad (\text{natural frequency}), \\ \xi_y &= \frac{c_y}{2\sqrt{k_y m}} \quad (\text{damping ratio}). \end{aligned} \tag{12}$$

For the one-degree-of-freedom torsional mode, Eq. (3) becomes

$$I_\theta \ddot{\theta} + c_\theta \dot{\theta} + k_\theta \theta = M_\theta, \tag{13}$$

where I_θ is the mass moment of inertia, c_θ is the torsional damping constant, k_θ is the torsional stiffness constant, and θ denotes the rotational angle of the body around the elastic centre. The right-hand side of Eq. (13) is the moment about the elastic centre. The nondimensional form of Eq. (13) is

$$\ddot{\theta} + 2\xi_\theta \left(\frac{2\pi}{U_\theta}\right) \dot{\theta} + \left(\frac{2\pi}{U_\theta}\right)^2 \theta = \frac{M^*}{n_\theta} = \frac{C_M}{2n_\theta}, \tag{14}$$

where

$$\begin{aligned} U_\theta &= \frac{U}{f_\theta D} \quad (\text{reduced velocity}), \\ n_\theta &= \frac{I_\theta}{\rho D^4} \quad (\text{mass moment of inertia ratio}) \end{aligned} \tag{15}$$

and

$$f_\theta = \frac{1}{2\pi} \sqrt{\frac{k_\theta}{I_\theta}} \quad (\text{natural frequency}),$$

$$\xi_\theta = \frac{c_\theta}{2\sqrt{k_\theta I_\theta}} \quad (\text{damping ratio}). \quad (16)$$

We note that f_θ is the structural natural frequency for rotational motion. For the remainder of this paper, the structural equations will be used in their nondimensionalized form.

In general, if we denote the vibrational amplitude of a particular model by A , then the dimensionless amplitude of flow-induced vibration A/D can be regarded as a function of four dimensionless parameters: (a) Reynolds number Re , as defined in Eq. (6); (b) reduced velocity $U_{red} = U/(f_n D)$, where f_n is the natural rotational or transverse structural frequency; (c) mass ratio or mass moment of inertia ratio denoted by n ; (d) damping factor ζ .

We may write this function as

$$\frac{A}{D} = \Theta(Re, U_{red}, n, \zeta). \quad (17)$$

The most important task in the study of fluid/structure interaction is to determine the relationship between the dimensionless amplitude and the governing dimensionless parameters.

If we consider a series of rectangular cross-section bodies with varying thickness ratios $A = L/D$, where L is the length of the body and D its thickness, then Eq. (17) will also include a geometric parameter and becomes

$$\frac{A}{D} = \Theta(A, Re, U_{red}, n, \zeta). \quad (18)$$

4. Quasi-steady analytical formulation

The theory related to galloping is based on a quasi-steady assumption; i.e., the fluid force on the structure is assumed to be determined solely by the instantaneous relative velocity and the angle of attack of the body (Blevins, 1990). Strictly, this requirement is only satisfied at high reduced velocity, $U_{red} > 20$ (Bearman et al., 1987), where the vortex-shedding frequency is significantly greater than the natural frequency of the body. However, large amplitude responses do occur for lower reduced velocities (Blevins, 1990), for cases where the combined mass and damping is low. This response is generally caused by vortex shedding or some combination of vortex shedding and galloping. In order to capture pure galloping instabilities, we confine our numerical experiments to higher reduced velocities $U_{red} \geq 20$ in order to avoid the effect of vortex-induced resonance. It has been found that the thickness ratio of rectangular sections plays an important role in the mechanism of instability (Washizu and Ohya, 1978; Washizu et al., 1980; Blevins, 1990; Nakamura and Tomonari, 1977). For example, in their free oscillation experiment, Nakamura and Tomonari (1977) found that soft transverse galloping, the spontaneous build up of oscillations from rest, only occurs for sections longer than a critical thickness ratio of 0.67. In this paper, the response of a range of rectangular sections at zero incidence with thickness ratios from 1 to 5 is investigated. The effect of increasing damping factor and mass ratio will also be considered.

4.1. Heaving/transverse mode

The following quasi-steady analysis to evaluate the aerodynamic damping on a heaving or transversely moving structure has been previously performed by many authors (Blevins, 1990; den Hartog, 1956) and is included here for

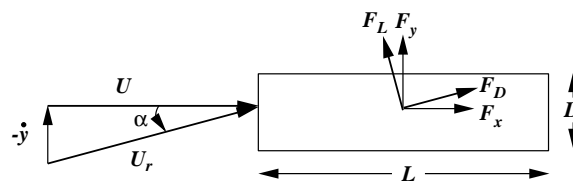


Fig. 1. Effective angle of incidence on a rectangular body.

completeness and to stimulate interest in the subsequent numerical experiments. Fig. 1 illustrates a body oscillating in a vertical direction under the influence of forces generated by a horizontal steady fluid flow of velocity U . Transverse response of the body results in a velocity relative to the structure of U_r , at an effective angle of incidence α , where

$$U_r^2 = U^2 + \dot{y}^2 \tag{19}$$

and

$$\tan \alpha = -\frac{\dot{y}}{U}. \tag{20}$$

Following aeronautical practice, the angle of incidence is positive for a clockwise rotation of the body in a flow travelling from left to right.

The force on the body per unit length is

$$F_y = \frac{1}{2} \rho U^2 D C_y, \tag{21}$$

where C_y is the vertical or normal force coefficient which is related to the lift and drag coefficients, C_L and C_D respectively, by

$$C_y = \frac{U_r^2}{U^2} (C_L \cos \alpha + C_D \sin \alpha). \tag{22}$$

Due to the nature of the quasi-steady analysis the normal force, lift and drag coefficients used in Eq. (22) should be taken as time-averaged values, where the variation of these coefficients due to high-frequency effects are ignored. Though it is not explicitly illustrated, subsequent appearances of these force coefficients should be read as time-averaged values. For small effective angles of attack, Eq. (20) can be approximated by

$$\alpha = -\frac{\dot{y}}{U}, \tag{23}$$

and C_y can be expanded about $\alpha = 0$, resulting in the relationship,

$$C_y \cong C_y \Big|_{\alpha=0} + \frac{\partial C_y}{\partial \alpha} \Big|_{\alpha=0} \alpha. \tag{24}$$

Assuming $C_y|_{\alpha=0} = 0$ and using Eq. (23) gives

$$C_y \cong -\frac{\partial C_y}{\partial \alpha} \Big|_{\alpha=0} \frac{\dot{y}}{U}. \tag{25}$$

By utilizing the representation of C_y in Eq. (25), we can formulate an expression for the vertical motion of the body,

$$\ddot{y}^* + 2\xi_y \left(\frac{2\pi}{U_y}\right) \dot{y}^* + \left(\frac{2\pi}{U_y}\right)^2 y^* = -\frac{1}{2n_y} \frac{\partial C_y}{\partial \alpha} \Big|_{\alpha=0} \dot{y}^*. \tag{26}$$

Considering Eq. (26) the net damping of the system, Ψ_y , can be shown to be

$$\Psi_y = 2\xi_y \left(\frac{2\pi}{U_y}\right) + \frac{1}{2n_y} \frac{\partial C_y}{\partial \alpha} \Big|_{\alpha=0}, \tag{27}$$

where $\Psi_y < 0$ will result in galloping oscillations, since the solution of Eq. (26) would be associated with exponentially growing oscillatory solutions. The structural damping, ξ_y , is always expected to be positive, therefore a necessary condition for instability is

$$\frac{\partial C_y}{\partial \alpha} \Big|_{\alpha=0} < 0. \tag{28}$$

4.2. Torsional/rotational mode

Similar to the analysis of the transverse mode, the net damping on a rotating structure, can be approximately evaluated using quasi-steady theory. Again this analytical investigation has been performed previously and the subsequent derivation mainly follows that of Blevins (1990). As we have seen, the relative angle of attack of a body vertically oscillating is proportional to its vertical velocity, here it will be shown that for a rotating body the relative angle of attack is proportional to its physical angle of attack, θ , and its rotational velocity, $\dot{\theta}$.

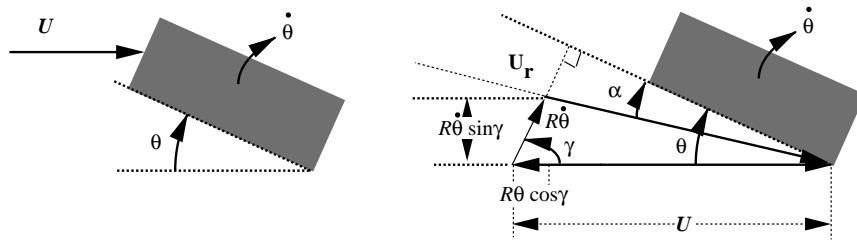


Fig. 2. Analytical model used in quasi-steady analysis of rotational galloping.

The relative angle of attack, α and relative velocity, U_r , at a radius R from the centre of rotation of a rotating body can be evaluated as

$$\alpha = \theta - \tan^{-1} [R\dot{\theta} \sin \gamma / (U - R\dot{\theta} \cos \gamma)], \tag{29}$$

$$U_r^2 = (R\dot{\theta} \sin \gamma)^2 + (U - R\dot{\theta} \cos \gamma)^2, \tag{30}$$

where the variables are defined in Fig. 2 and $\gamma = \pi/2 - \theta$. The effective angle of attack, α , is obviously dependent on the physical angle of attack, θ , though there is also a contribution due to the rotational velocity of the body.

The rotational motion at the leading edge of the body results in a decrease of the relative free-stream velocity in the horizontal direction. Therefore $(U - R\dot{\theta} \cos \gamma)$ is the velocity of the horizontal free stream when the rotational velocity of the body is considered. The vertical motion of the leading edge can be related to the quasi-steady theory for purely heave or transverse motion where a nondimensional positive transverse velocity was shown to be equivalent to a negative effective angle of attack. The leading edge of the body, for small oscillations about a mean displacement, can in a linear sense be said to be undergoing purely vertical motion. As with the transverse theory this vertical motion can be related to an effective angle of attack.

The length R is a characteristic radius at which the relative angle of attack due to the rotational velocity of the body is considered. This technique has been used extensively for many different geometries (Blevins, 1990; Nakamura and Mizota, 1975; Sisto, 1953) and in the case of rectangular sections, R is taken to be the half-chord length, $D/2$ (Blevins, 1990; Nakamura and Mizota, 1975). This choice of R results in the instantaneous relative angle of attack, α , being evaluated at the leading edge where the most significant forces for rotational motion are generated. For small angles and velocity, Eqs. (29) and (30) reduce to

$$\alpha \approx \theta - \frac{R\dot{\theta}}{U}, \tag{31}$$

$$U_r \approx U. \tag{32}$$

Therefore the time-averaged value of C_M can be expanded about $\alpha = 0$, resulting in the relationship

$$C_M \cong \left. \frac{\partial C_M}{\partial \alpha} \right|_{\alpha=0} \alpha, \tag{33}$$

$$\cong \left. \frac{\partial C_M}{\partial \alpha} \right|_{\alpha=0} \left(\theta - \frac{R\dot{\theta}}{U} \right). \tag{34}$$

The same method adopted for the transverse galloping analysis is now applied to formulate a condition which will determine the potential of a body to undergo rotational galloping. An equation of motion governing the rotation of the body can be formulated by combining the structural equation (14) and the expansion of C_M in Eq. (34) to arrive at

$$\ddot{\theta} + 2\zeta_\theta \left(\frac{2\pi}{U_\theta} \right) \dot{\theta} + \left(\frac{2\pi}{U_\theta} \right)^2 \theta = \frac{1}{2n_\theta} \left. \frac{dC_M}{d\alpha} \right|_{\alpha=0} (\theta - R^*\dot{\theta}), \tag{35}$$

where $R^* = R/D$. Considering Eq. (35) the net damping of the system, Ψ_θ , is

$$\Psi_\theta = 2\zeta_\theta \left(\frac{2\pi}{U_\theta} \right) + \frac{R^*}{2n_\theta} \left. \frac{dC_M}{d\alpha} \right|_{\alpha=0}. \tag{36}$$

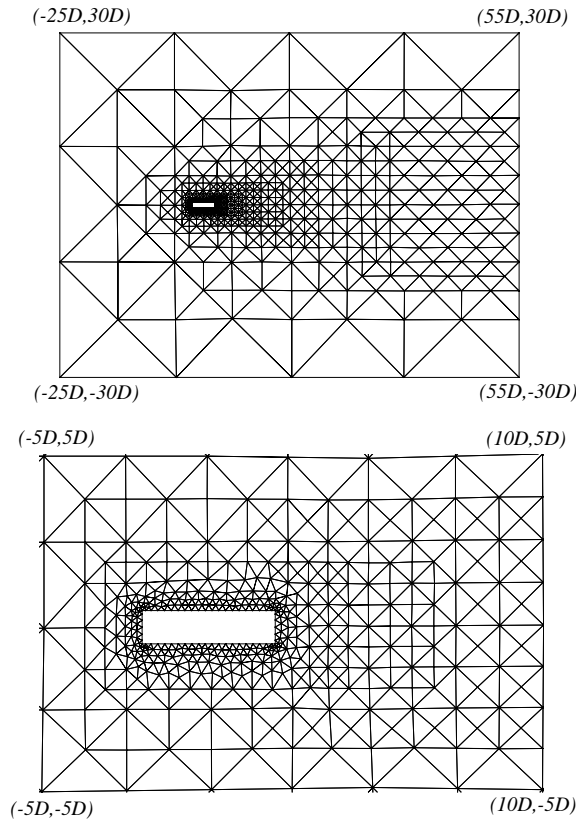


Fig. 3. Mesh configuration for numerical experiments.

The condition $\Psi_\theta < 0$ will result in rotational galloping oscillations. As the structural damping for rotational motion, ξ_θ , is always expected to be positive, a necessary condition for instability under this linearized analysis is

$$\left. \frac{\partial C_M}{\partial \alpha} \right|_{\alpha=0} < 0 \tag{37}$$

when $\alpha \ll 1$.

5. Numerical investigation of transverse galloping

To enable a prediction of the instability of a series of rectangular sections with increasing thickness ratio A , a number of numerical experiments were conducted to evaluate the force coefficients C_y , for a range of angles of attack. The full computational mesh and an enlargement centred on the body for a rectangular structure with $A = 4$ can be seen in Fig. 3. The mesh consists of 1303 high-order sub-domains and the computations were carried out using a polynomial expansion basis of order 5. Spatial and temporal convergence tests for an oscillating section can be found in Appendix A.

The results for thickness ratios ranging from 1 to 5 can be seen in Fig. 4, where C_y is represented as a function of α . The values of $dC_y/d\alpha|_{\alpha=0}$ are contained in Table 1.

According to the quasi-steady theory, if the variation of force coefficient with angle of attack has a negative slope around the origin, the corresponding structure is potentially unstable to transverse galloping. Therefore by studying the results in Fig. 4 and Table 1, the structures with thickness ratios $A = 1$ (square section), 1.5 and 2 may be susceptible to transverse galloping. To test this hypothesis these cases were simulated numerically by subjecting the sections to a fluid flow parallel to the x -axis and allowing them to oscillate with fluid and structural parameters set to: $\xi_y = 0.0037$, $U_y = 40$, $n = 20$ and $Re = 250$.

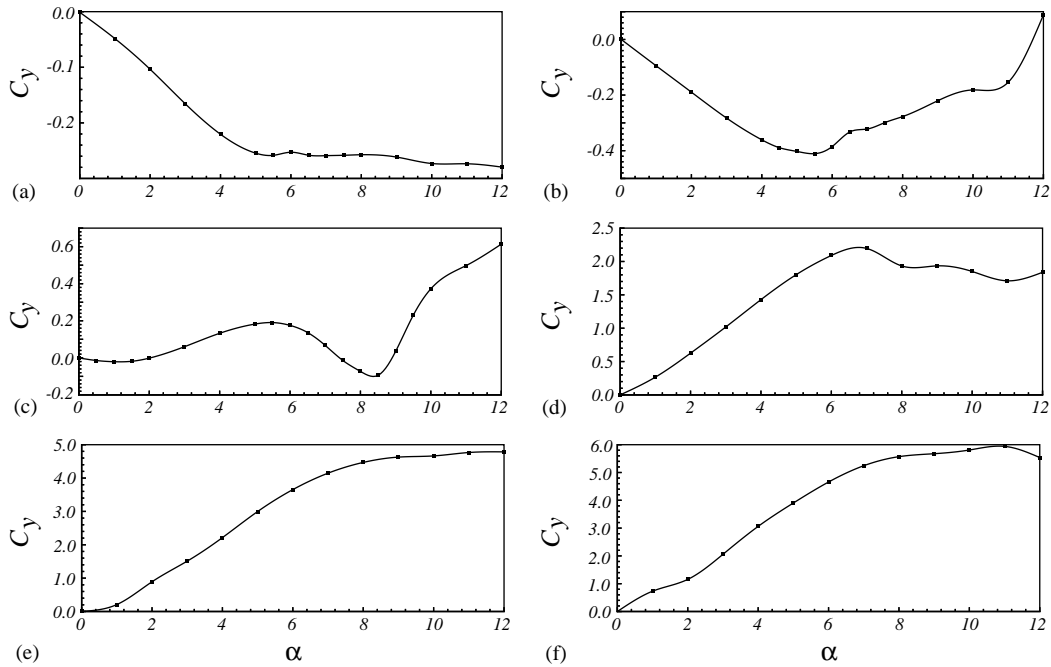


Fig. 4. Lift force coefficients against the angle of attack α (deg.) for different thickness ratios (A) of rectangle cylinders. (a) $A = 1$, (b) $A = 1.5$, (c) $A = 2$, (d) $A = 3$, (e) $A = 4$, (f) $A = 5$.

Table 1

Variation of the lift coefficient against angle of attack in radians about a zero effective angle of attack for rectangular sections of increasing aspect ratio

A	1	1.5	2	3	4	5
$\frac{dC_y}{d\alpha} _{\alpha=0}$	-2.69	-5.44	-1.60	14.4	7.39	44.8

The results are presented in Fig. 5, where as expected the rectangular sections with thickness ratios $A = 1$ and 1.5 experience large-amplitude oscillations (approximately 100% of the thickness of the cylinder). From the plots shown in Figs. 4(a) and (b), we see that as the angle of attack increases, it eventually enters a stable range, from the point of view of the quasi-steady analysis. Therefore we expect an increase in effective angle of incidence to lead to a reduction in the magnitude of the applied aerodynamic force. Applying the quasi-steady analysis in this manner suggests a mechanism as to why the large oscillation amplitudes due to the galloping instability can be sustained without growing unbounded.

For a thickness ratio of $A = 2$, the amplitude of the oscillations is reduced (approximately 0.015% of the thickness of the cylinder), though low-frequency oscillations are observed, indicating a galloping motion. Superimposed on this low-frequency modulation is a high-frequency motion due to vortex shedding. The low recorded amplitude is possibly due to the fact that the instability range and magnitude of $dC_y/d\alpha$ for this case is very small, see Fig. 4(c) and Table 1.

For larger A , transverse galloping is suppressed, as predicted by the quasi-steady theory. These numerical results are consistent with the experimental results of Washizu and Ohya (1978), which show that no transverse galloping exists for $A > 2.5$. At higher values of A only vortex-induced oscillations are observed, which is illustrated in the response time history plotted in Fig. 5(d). The agreement between the experimental results of Washizu et al. (1980) and the current numerical results may be surprising when considering the disparity in Reynolds number. The experiments were conducted at a Reynolds number of $\approx 1 \times 10^5$ while the numerical results were performed at $Re = 250$. However, we

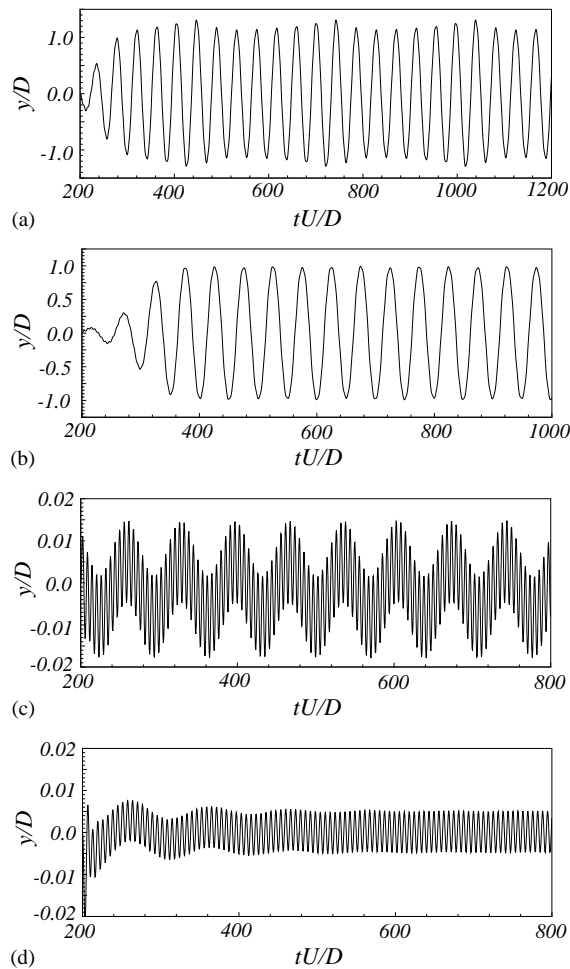


Fig. 5. Transverse galloping responses: $Re = 250$, $\xi_y = 0.0037$, $U_y = 40$, $n_y = 20$. (a) $A = 1$, (b) $A = 1.5$, (c) $A = 2$, (d) $A = 3$.

note that sharp-edged bodies such as the rectangular sections have well-defined separation points which show no significant movement for changes in Reynolds number. The reattachment of the separated shear layers is Reynolds number dependent, though it appears from the agreement between the experimental and numerical results that this difference has little effect on the stability or otherwise of the sections. Another significant difference between the experimental and numerical results is the restriction of the numerical experiments to two-dimensional flow. It has been shown that this restriction has some effect on the quantitative comparison of the amplitude of the unstable motions, but less effect on the point of instability (Robertson et al., 2002, 2001). At low oscillation amplitudes, that is those prior to the galloping instability, the vortical structures produced experimentally may have a finite correlation length implying a decorrelation of the forces and moments along the length of the bridge deck section. It has been shown for circular cylinders that when the oscillations increase in size the vortical structures become homogeneous along the length of the body (Kozakiewicz et al., 1992; Sumer et al., 1994). In summary, the three-dimensionality of the physical experiments can have an effect on the quantitative oscillation amplitudes, without affecting the reduced velocity for the onset of galloping.

To investigate the effect of increasing reduced velocity on galloping, the motion of a section with $A = 1$ was calculated at three values of reduced velocity. The time histories of the amplitude are shown in Fig. 6. The results indicate an increase of amplitude with a corresponding increase in reduced velocity. This is consistent with the quasi-steady linear theory, Eq. (27), which predicts a decrease in the aerodynamic damping for an increase in the reduced velocity. Though it must be remembered that Eq. (27) does not allow us to predict the amplitude of the galloping oscillations, it can be inferred that, for a given body, a reduction in the damping may lead to an increase of the amplitude of the oscillations.

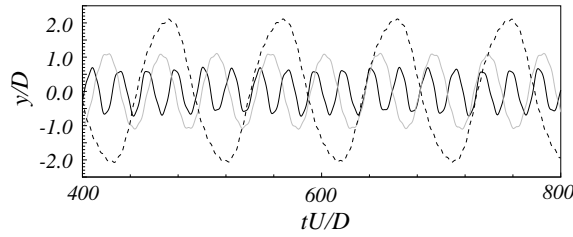


Fig. 6. Transverse galloping responses: $Re = 250$, $\zeta_y = 0.0037$, $A = 1$, $n_y = 10$. (a) —, $U_y = 20$, (b), $U_y = 40$, (c) - - -, $U_y = 80$.

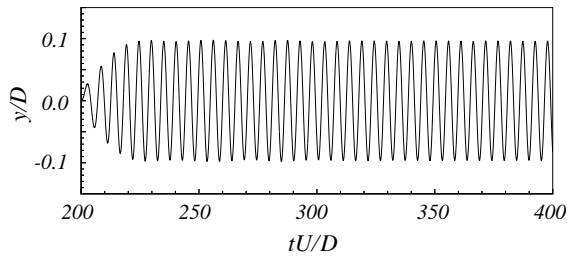


Fig. 7. Vortex-induced oscillation: $Re = 250$, $\zeta_y = 0.0037$, $U_y = 6.0$, $n_y = 10$, $A = 1.5$.

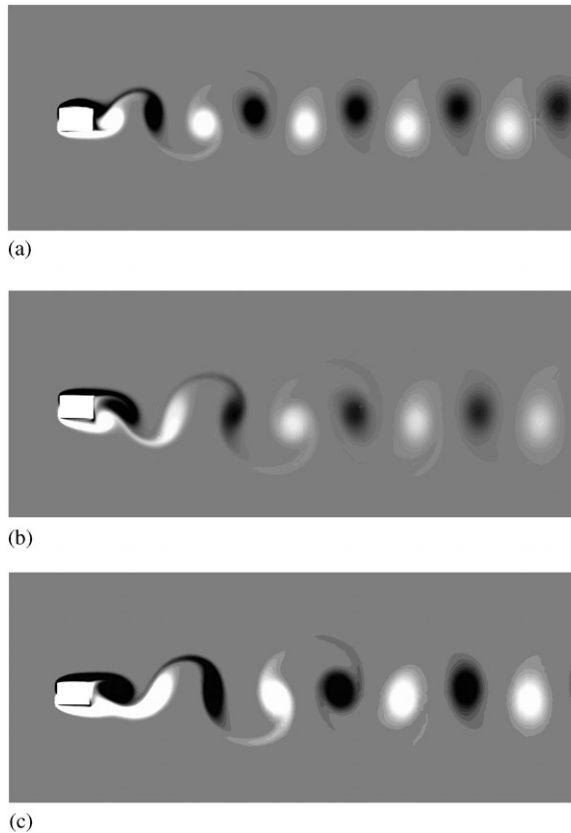


Fig. 8. Comparison of flow patterns for vortex-induced response, transverse galloping and fixed rectangular sections: $Re = 250$, $\zeta_y = 0.0037$, $A = 1.5$, $n_y = 10$. (a) Vortex-induced resonance, $U_y = 6$, (b) transverse galloping, $U_y = 40$, (c) fixed.

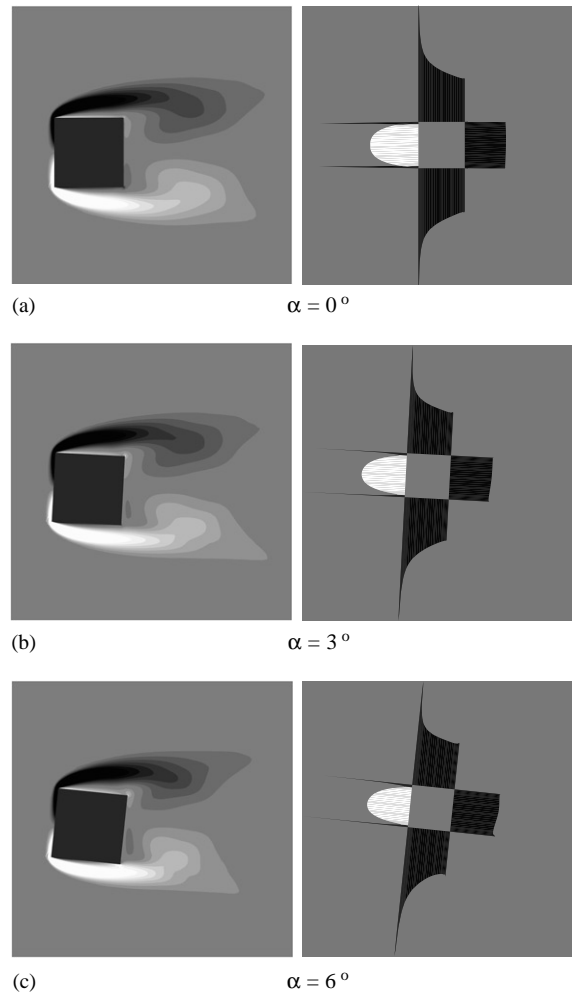


Fig. 9. Averaged vorticity field, pressure distribution (white indicating positive pressure and black negative) for a fixed square section at $Re = 250$. (a) $\alpha = 0^\circ$; (b) $\alpha = 3^\circ$; (c) $\alpha = 6^\circ$.

The quasi-steady theory predictions are only valid for small oscillations about the origin, as the amplitude of the oscillations increases nonlinear flow behaviour not accounted for in the quasi-steady analysis may lead to the saturation of the large amplitude oscillations. Therefore, whilst the quasi-steady theory predicts that the oscillations will become unbounded in time, it can be seen from Fig. 6 that the oscillations are still bounded due to nonlinear flow behaviour up to at least a reduced velocity of $U_y = 80$.

A comparison between galloping and vortex excited resonance was also performed for the rectangular section of thickness ratio equal to 1.5. The Strouhal number for this section was estimated to be 0.158. Therefore we considered a reduced velocity of $1/0.158 \approx 6$, which produced vortex lock-in oscillations as can be seen in Fig. 7. For this resonant case, the oscillation amplitude is much smaller than those recorded for galloping at higher reduced velocities. A comparison of the flow patterns for vortex-induced resonance, galloping and a fixed structure is illustrated in Fig. 8, where it is apparent that the longitudinal spacing between vortices is reduced for the vortex lock-in case when compared to the galloping and fixed structure cases. This illustrates the quasi-steady nature of the flow for the galloping structure since the vortex pattern and spacing are very similar to that of the fixed body, whereas the unsteady, high-frequency vortex-induced motion exhibits a decrease in spacing of the shed vortices.

The numerical results generated here are in agreement with the theory proposed by Parkinson (1971) to explain the transverse (or heave) galloping mechanism. He theorized that the motion of a rectangular structure led to the separated shear layers above and below the section becoming asymmetric. The closely lying shear layer produces a greater suction

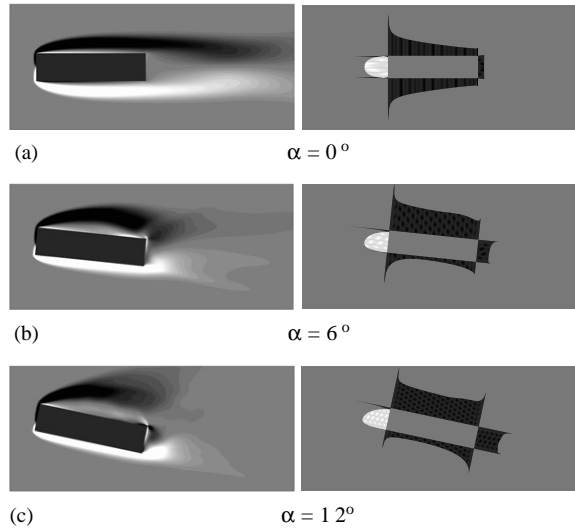


Fig. 10. Averaged vorticity field, pressure distribution (white indicating positive pressure and black negative) for a fixed rectangular section of $A = 4$ at $Re = 250$. (a) $\alpha = 0^\circ$; (b) $\alpha = 6^\circ$; (c) $\alpha = 12^\circ$.

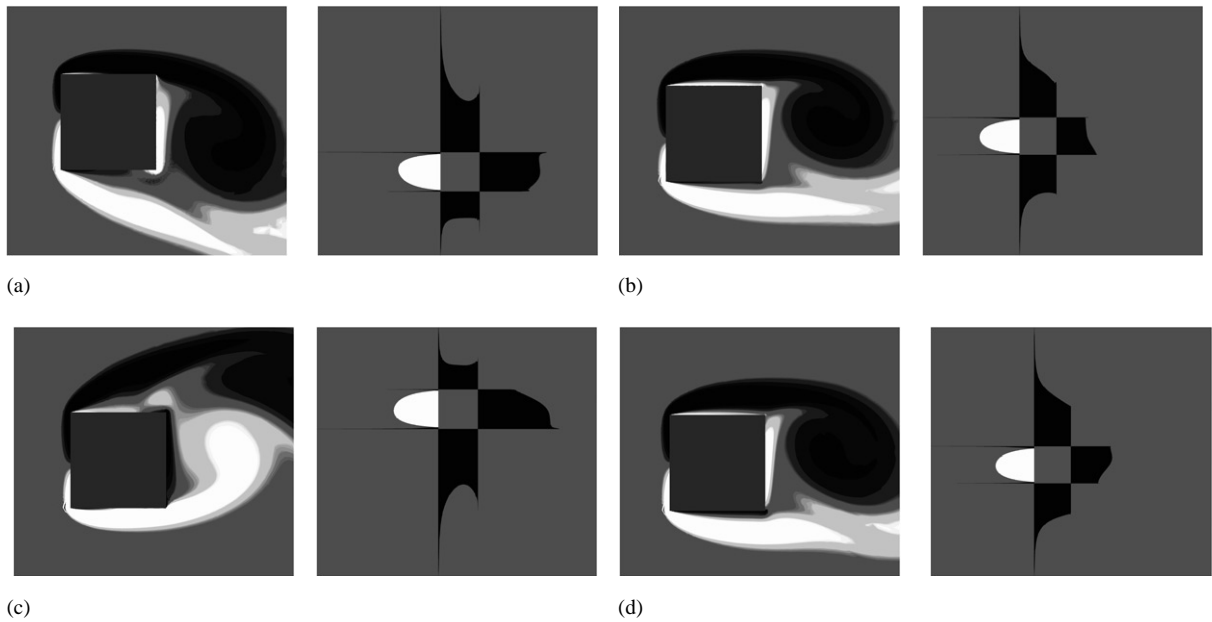


Fig. 11. Vorticity field, pressure distribution (white indicating positive pressure and black negative) and displacement time-history of galloping rectangular section: $A = 1$, $Re = 250$, $\xi_y = 0.0037$, $n_y = 10$, $U_y = 40$.

on the body than a more distant layer and therefore a pressure difference is formed across the body. Fig. 9 shows the time-averaged flow field and pressure distribution, in terms of C_p , around the square section for increasing fixed angles of attack, α . The shear layers are asymmetric when $\alpha = 3^\circ$, causing a pressure difference which is not immediately apparent from Fig. 9(b), but is implied by the variation of C_y with α shown in Fig. 4(a). The pressure applied on the bottom edge of the section is of a higher magnitude than that applied on the top edge. An increase to 6° leads to a larger pressure difference due to the greater asymmetry of the shear layers. Since $\alpha = -\dot{y}/U$, the motion of the body leads to an asymmetry of the shear layers and a force in the direction of the motion of the body, thereby causing large amplitude, unstable oscillations.

For higher aspect ratio rectangular sections, $A > 2.5$, the top shear layer reattaches or nears reattachment for positive displacements about $\alpha = 0$. This results in a pressure difference which exerts a force in the opposite direction to the motion of the section. The aerodynamic damping is therefore positive and the oscillations are stable. The reattaching shear layers and pressure differential are illustrated in Fig. 10 for a rectangular section of $A = 4$ and $Re = 250$ at various effective angles of attack α .

Fig. 11 shows the vorticity field around a moving rectangular section of aspect ratio $A = 1$ undergoing a vertical galloping motion, and the resulting pressure distribution in terms of C_p . The displacement time history of the section is also shown in Fig. 11, with the position of the body at the various snapshots highlighted. It can be seen that at the points (b) and (d) the body is close to its maximum displacement and therefore \dot{y} and α are approximately zero. Consequently, the shear layers are predominantly symmetric, resulting in a minimal force acting on the body. In contrast to this the asymmetric vorticity fields in Figs. 11(a) and (c), where α is negative and positive, respectively, indicate the possibility of a pressure difference acting in the direction of motion. Therefore, the structure is taking energy from the fluid, resulting in unstable, growing oscillations.

6. Numerical investigation of rotational galloping

The previous numerical experiments undertaken in Section 5 to evaluate $\partial C_y / \partial \alpha$ also provide the variations of C_M against α . These results are shown in Fig. 12 and will be compared with the dynamic numerical simulations discussed

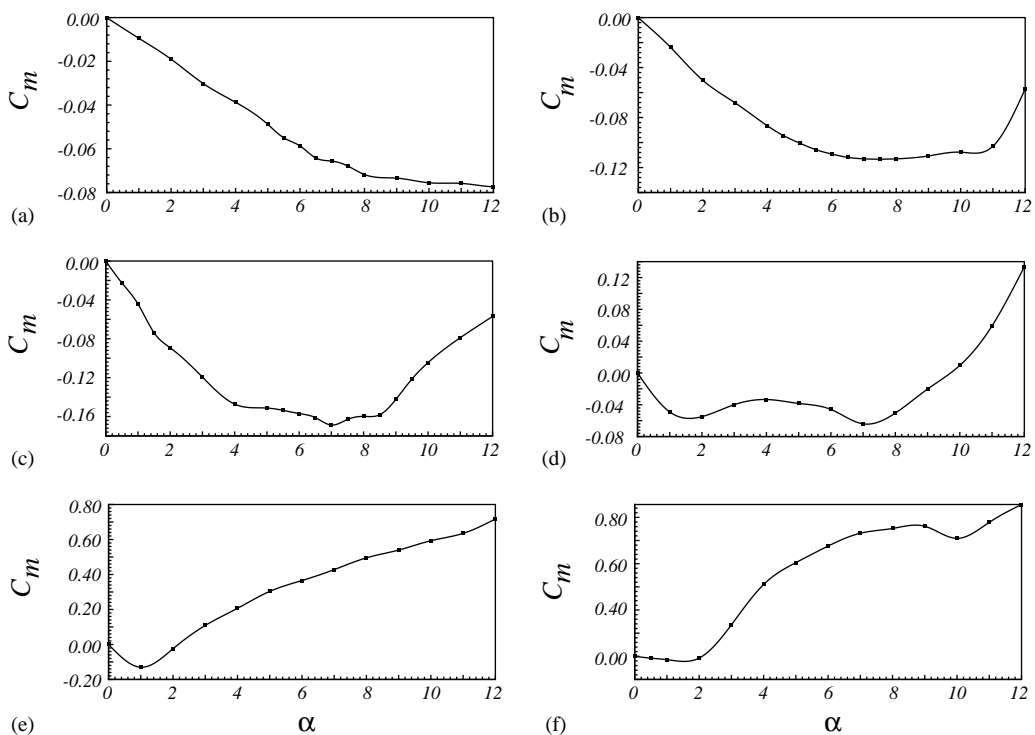


Fig. 12. Moment coefficient, C_M , against angle of attack α (deg). (a) $A = 1$, (b) $A = 1.5$, (c) $A = 2$, (d) $A = 3$, (e) $A = 4$, (f) $A = 5$.

Table 2

Variation of the moment coefficient against angle of attack in radians about a zero effective angle of attack for rectangular sections of increasing aspect ratio

A	1	1.5	2	3	4	5
$\frac{dC_M}{d\alpha} _{\alpha=0}$	-0.516	-1.49	-2.46	-3.21	-9.74	-2.06

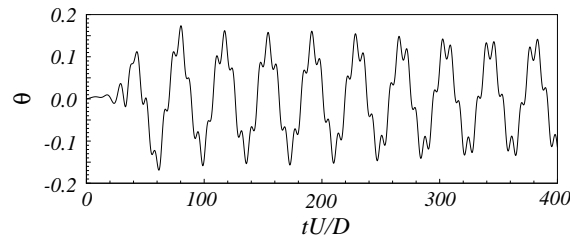


Fig. 13. Free rotation: $A = 1$, $\xi_\theta = 0$, $U_\theta = 40$, $n_\theta = 100$.

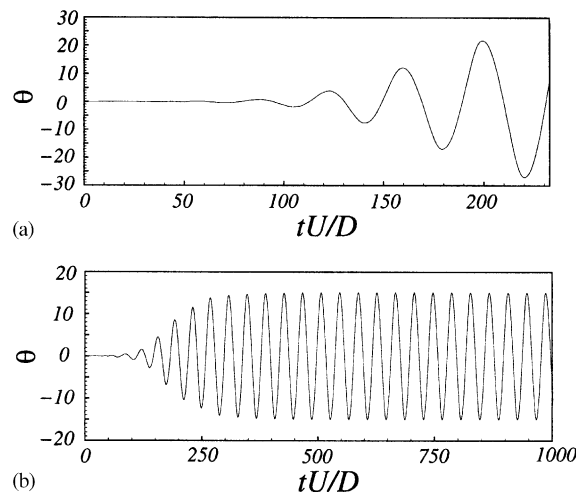


Fig. 14. Free rotation: $Re = 250$, $A = 2$, $U_\theta = 40$, $n_\theta = 100$; (a) $\xi_\theta = 0$, (b) $\xi_\theta = 0.1$.

below. The values of $dC_M/d\alpha|_{\alpha=0}$ are also given in Table 2. The data in Fig. 12 and Table 2 indicate that all the sections considered have the potential to gallop as the value of $dC_M/d\alpha$ about $\alpha = 0$ is negative for all cases. This is consistent with the findings of other authors (Blevins, 1990; Nakamura and Mizota, 1975; Luo et al., 1998), who consequently went on to show experimentally that bodies with $dC_M/d\alpha < 0$ are dynamically unstable.

The dynamic numerical experiments were performed by allowing the section to rotate when acted upon by an oncoming free-stream parallel with the x -axis. For the square section, $A = 1$, we see from Fig. 13 that there are two distinct frequencies appearing in the time history of the nondimensional oscillation amplitude, the high frequency relates to the oscillating force produced by the vortex shedding, and the low frequency indicates a galloping motion. The low amplitude of the galloping oscillations is expected from the very small value of $dC_M/d\alpha$ for this geometry.

For all sections of aspect ratio greater than one, the numerical results in Figs. 14–17 show large-amplitude galloping oscillations for a reduced velocity of 40. For the cases with no structural damping, $\xi_\theta = 0$, the amplitude appears to be

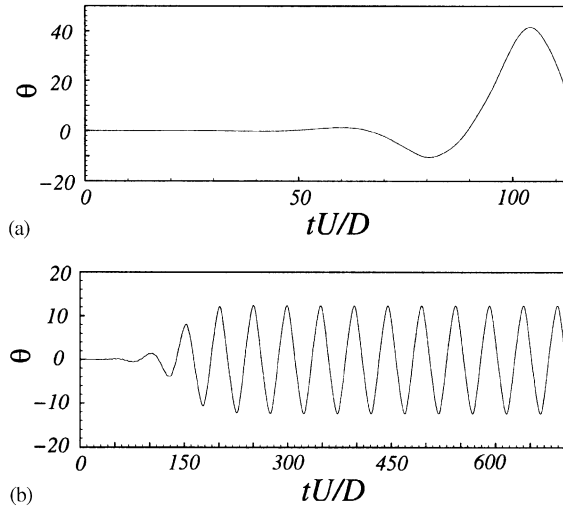


Fig. 15. Free rotation: $Re = 250$, $\Lambda = 3$, $U_\theta = 40$, $n_\theta = 100$; (a) $\xi_\theta = 0$, (b) $\xi_\theta = 0.5$.

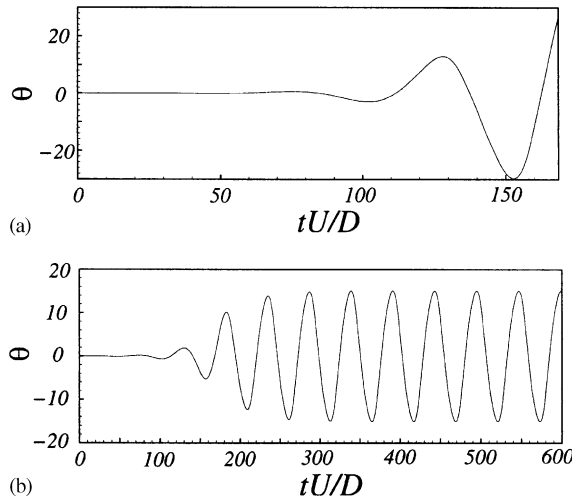


Fig. 16. Free rotation: $Re = 250$, $\Lambda = 4$, $U_\theta = 40$, $n_\theta = 400$; (a) $\xi_\theta = 0$, (b) $\xi_\theta = 0.25$.

unbounded. However, this cannot be confirmed due to the limitations of the computational code, which is not valid for very high amplitudes of displacement due to the implementation of the boundary conditions (Li et al., 2002). The galloping oscillations have been limited by increasing the damping ratio, ξ_θ , to a sufficiently large value, as seen in subplots (b) of Figs. 14–17.

The motion of a rectangular section with $\Lambda = 4$, $n_\theta = 400$, $\xi_\theta = 0.25$, $U_\theta = 40$ and $Re = 250$ can be seen in Fig. 18, where the flow field is represented as vorticity contours and the pressure distribution given in terms of C_p . The large pressure variations at time levels (a) and (c) occur when the rotational displacement is at its greatest and together with the time history of the displacement in Fig. 18(e) it can be seen that the moment and displacement are largely in phase. However, it is the component of the moment which is in phase with the velocity that causes the instability of the motion. The vorticity fields associated with the component of the moment in the direction of motion can be seen at time levels (b) and (d), where it is apparent that the pressure distribution is such that the moment of the section is in the direction of motion of the body, thus causing unstable oscillations.

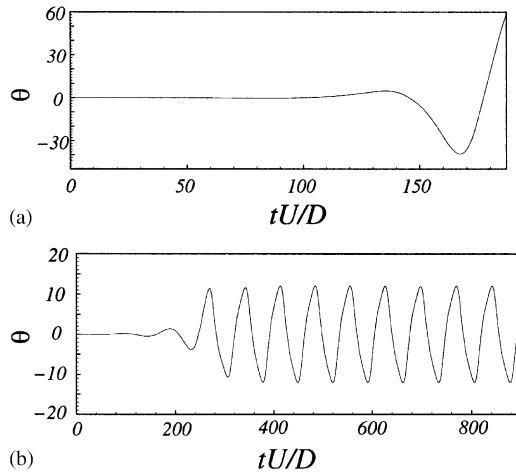


Fig. 17. Free rotation: $Re = 250$, $\Lambda = 5$, $U_\theta = 40$, $n_\theta = 400$; (a) $\xi_\theta = 0$, (b) $\xi_\theta = 0.6$.

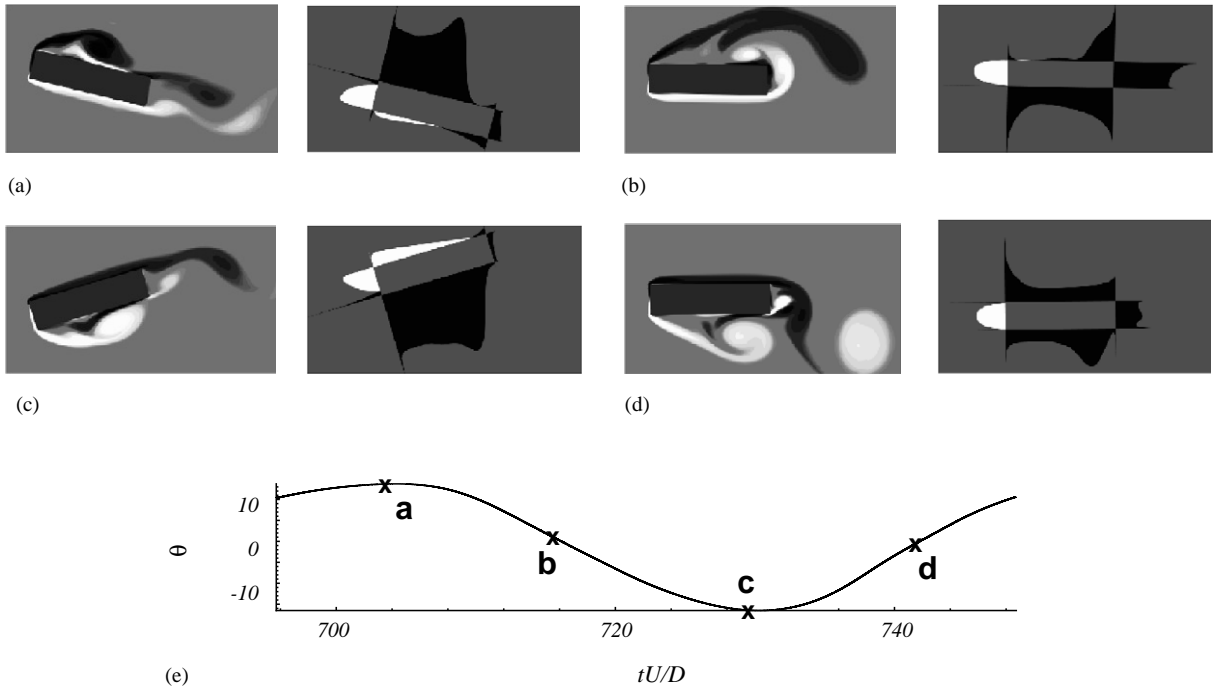


Fig. 18. Vorticity field, pressure distribution (white indicating positive pressure and black negative) and displacement time-history of galloping rectangular section: $\Lambda = 4$, $Re = 250$, $\xi_\theta = 0.25$, $n_\theta = 400$, $U_\theta = 40$.

7. Conclusions

We have reviewed the quasi-steady analysis of transverse and rotational galloping and presented numerical simulations which dynamically predict the galloping modes. The numerical results for both transverse and rotational galloping are consistent with the predictions of quasi-steady analysis although only a qualitative agreement is observed.

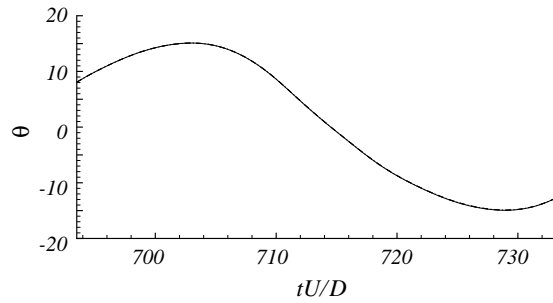


Fig. 19. Time history of rotational displacement for: —, $\Delta t = 0.0005$; - - -, $\Delta t = 0.001$; and - · - · -, $\Delta t = 0.002$.

Table 3

Temporal convergence of the displacement of a rotating body with an aspect ratio of $A = 4$ at $tU/D = 725$

Δt	θ	% difference to $\Delta t = 0.0005$
0.002	13.4933	0.42
0.001	13.5209	0.21
0.0005	13.5497	0

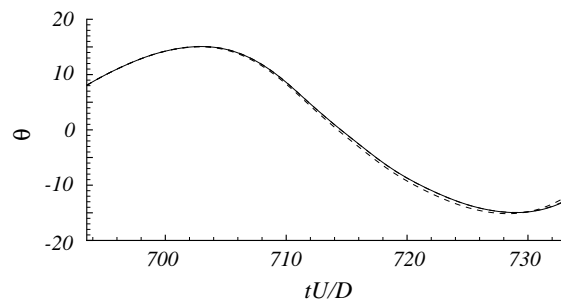


Fig. 20. Time history of rotational displacement for: —, $p = 6$; - · - · -, $p = 4$; and - - -, $p = 2$.

Table 4

Spectral element spatial convergence of the displacement of a rotating body with an aspect ratio of $A = 4$ at $tU/D = 725$

Polynomial order (P)	θ	% difference to $P = 6$
2	14.0421	3.6
4	13.5497	0.12
6	13.5331	0

Much of the work is in qualitative agreement with experimental data (Parkinson, 1971; Blevins, 1990; Washizu and Ohya (1978)) obtained at a higher Reynolds numbers ($\approx 1 \times 10^5$) than the current numerical data ($= 250$). This observation may well be related to the Reynolds number independent separation point of the rectangular structures considered although we note that the reattachment length of a separated shear layer is Reynolds number dependent (Parkinson, 1971).

To conclude, a detailed analysis of low Reynolds number flow past different aspect ratio rectangular cylinders which are allowed to either translate vertically or rotate has been performed.

Acknowledgements

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Appendix A. Spatial and temporal convergence

Spatial and temporal convergence is illustrated for the case of a rectangular body with an aspect ratio $A = 4$ undergoing large-amplitude rotational galloping motion. The flow and structure are described by the following parameters: $A = 4$, $n_\theta = 400$, $\xi_\theta = 0.25$, $U_\theta = 40$ and $Re = 250$. Time histories of the rotational displacement for increasing computational time increments are shown in Fig. 19. The three time histories are indistinguishable, with the two smallest time increments differing by approximately 0.2%. The percentage error between the results can be seen in Table 3.

Time histories of the rotational displacement for increasing polynomial order of the expansion basis are shown in Fig. 20. Spatial convergence is illustrated by the time histories when using polynomial orders of 4 and 6. The histories differ by approximately 0.1%. The percentage error between the results can be seen in Table 4.

References

- Anagnostopoulos, P., 1994. Numerical investigation of response and wake characteristics of a vortex-excited cylinder in a uniform stream. *Journal of Fluids and Structures* 8, 367–390.
- Bearman, P.W., Gartshore, I.S., Maull, D.J., Parkinson, G.V., 1987. Experiments of flow-induced vibrations of a square-section cylinder. *Journal of Fluids and Structures* 1, 19–34.
- Blevins, R.D., 1990. *Flow-Induced Vibration*. Van Nostrand Reinhold, New York.
- den Hartog, J.P., 1956. *Mechanical Vibrations*, 4th Edition. McGraw-Hill, New York.
- Deniz, S., 1997. Oscillating rectangular and octagonal profiles: interaction of leading- and trailing-edge vortex formation. *Journal of Fluids and Structures* 11, 3–31.
- Kozakiewicz, A., Sumer, B.M., Fredsoe, J., 1992. Spanwise correlation on a vibrating cylinder near a wall in oscillatory flows. *Journal of Fluids and Structures* 6, 371–392.
- Li, L., Sherwin, S.J., Bearman, P.W., 2002. A moving frame of reference algorithm for fluid/structure interaction of rotating and translating bodies. *International Journal of Numerical Methods in Fluids* 38, 207–223.
- Luo, S.C., Chew, Y.T., Lee, T.S., Yazdani, M.G., 1998. Stability to translational galloping vibration of cylinders at different mean angles of attack. *Journal of Sound and Vibration* 215, 1183–1194.
- Nakamura, Y., Mizota, T., 1975. Torsional flutter of rectangular prisms. *Journal of Engineering Mechanics Division ASCE EM2* (101), 125–142.
- Nakamura, Y., Nakashima, M., 1986. Vortex excitation of prisms with elongated rectangular, h and \vdash cross-sections. *Journal of Fluid Mechanics* 163, 149–169.
- Nakamura, Y., Tomonari, Y., 1977. Galloping of rectangular prisms in a smooth and a turbulent flow. *Journal of Sound and Vibration* 52, 233–241.
- Naudascher, E., Rochwell, D., 1994. *Flow-Induced Vibration—An Engineering Guide*. A.A. Balkema, Rotterdam.
- Nomura, T., 1993. Finite element analysis of vortex-induced vibrations of bluff cylinders. *Journal of Wind Engineering and Industrial Aerodynamics* 46, 587–594.
- Nomura, T., Hughes, T.J.R., 1992. An arbitrary Lagrangian–Eulerian finite element method for interaction of fluid and a rigid body. *Computer Methods in Applied Mechanics and Engineering* 95, 115–138.
- Parkinson, G., 1971. Wind-induced instability of structures. *Philosophical Transactions of the Royal Society of London A* 269, 395–409.
- Piperno, S., 1998. Numerical simulation of aeroelastic instabilities of elementary bridge decks. INRIA, Rapport de recherche, 3549th Edition.
- Robertson, I., Sherwin, S.J., Bearman, P.W., 2001. Prediction of flutter instability due to cross winds of the second forth road bridge. In: ECCOMAS 2001.
- Robertson, I., Sherwin, S.J., Bearman, P.W., 2002. Flutter instability prediction techniques for bridge deck sections. *International Journal for Numerical Methods in Fluids*, in press.
- Scanlan, R., 1979. On the state of stability considerations for suspended-span bridges under wind. In: *Proceedings IUTAM-IAHR Symposium, Karlsruhe, Germany*, pp. 595–618.
- Scanlan, R., Tomko, J., 1971. Airfoil and bridge deck flutter derivatives. *Journal of the Engineering Mechanics Division, ASCE EM6* (97), 1717–1737.
- Schulz, K., Kallinderis, Y., 1998. Unsteady flow structure interaction for incompressible flows using deformable hybrid grids. *Journal of Computational Physics* 143, 569–597.

- Sisto, F., 1953. Stall-flutter in cascades. *Journal of Aeronautical Science* 20, 598–604.
- Steinman, D.B., Watson, S.R., 1957. *Bridges and their Builders*. Dover, New York.
- Sumer, B.M., Fredsoe, J., Jensen, K., 1994. A note on spanwise correlations on a freely vibrating cylinder in oscillatory flow. *Journal of Fluids and Structures* 8, 231–238.
- Washizu, K., Ohya, A., 1978. Aeroelastic instability of rectangular cylinders in a heaving mode. *Journal of Sound and Vibration* 59, 195–210.
- Washizu, K., Ohya, A., Otsuki, Y., Fujii, K., 1980. Aeroelastic instability of rectangular cylinders in a torsional mode due to a transverse wind. *Journal of Sound and Vibration* 72, 507–521.
- Wei, R., Sekine, A., Shimura, M., 1995. Numerical analysis of 2D vortex-induced oscillations of a circular cylinder. *International Journal for Numerical Methods in Fluids* 21, 993–1005.